

Strain, deformation, stress

For fluids: have p, T to describe them.

No resistance to change shape

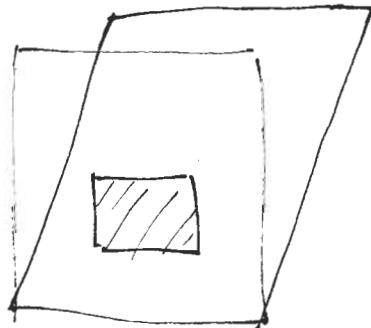
In solids pressure has to be replaced by elastic stresses: tensors.

First: address stress and strain

Let us look at a cube,

perform homogeneous, elastic deformation

- Shear
- Stretch

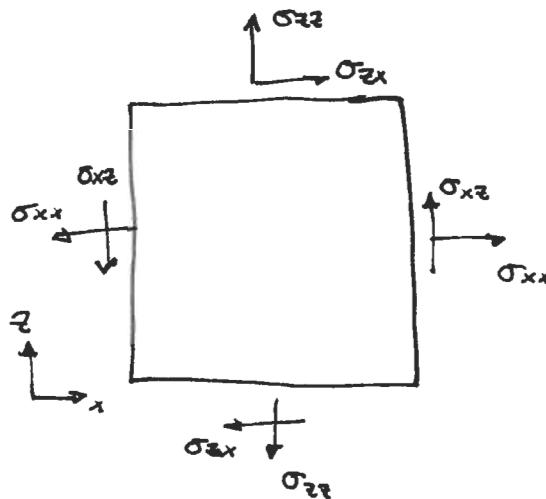


If some change remains when forces are unloaded: plastic deformation,  
otherwise: elastic deformation: perfectly restores original shape.

Let us cut out a small cube in the deformed material  
and keep forces on surface, so that it keeps its shape

Strain: force per area :

Orientation of force and area



We use the term

$A\sigma_{ij}$  & force acting on element

with i as normal vector and

j - component of this force

$A\sigma_{zz}$ : tensile force / strain

$A\sigma_{zx}$ : shear force / strain.

Torque:

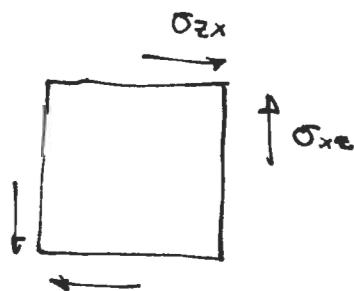
The volume element is not moving : stationary in elastic material

Therefore: no torque

In order for the volume element to be stationary, we require

$$\sigma_{xz} = \sigma_{zx}$$

$$\therefore \sigma_{ij} = \sigma_{ji}$$



This implies that  $\sigma_{ij}$  is a symmetric tensor.

In the dynamic case - the element may have an angular-acceleration

But : moment of inertia goes to zero faster than  
shear forces

unless the symmetry is fulfilled : will not get  
finite angular acceleration.

Therefore : general  $\sigma_{ij} = \sigma_{ji}$

This leaves us with 6 independent components

$$\begin{array}{c} \sigma_{xx} \quad \sigma_{yy} \quad \sigma_{zz} \\ \underbrace{\qquad\qquad\qquad}_{\text{tensile components}} \end{array} \quad \begin{array}{c} \sigma_{xy} \quad \sigma_{xz} \quad \sigma_{yz} \\ \underbrace{\qquad\qquad\qquad}_{\text{shear components}} \end{array}$$

The pressure is

$$p = -\frac{1}{3}(\sigma_{xx} + \sigma_{yy} + \sigma_{zz}) = -\frac{1}{3}\text{Tr}(\sigma_{ij})$$

This is an invariant : not dependent on choice of coord. system!



### Example

$$\text{Bar: } \sigma_{zz} = 0$$

$$\sigma_{xx} = \sigma_{xz} = 0$$

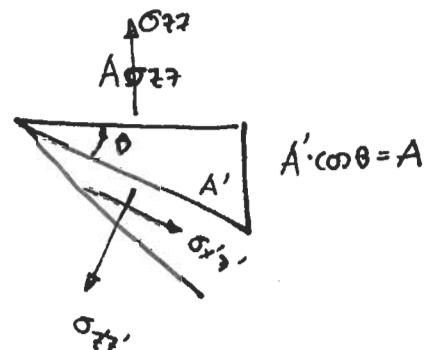
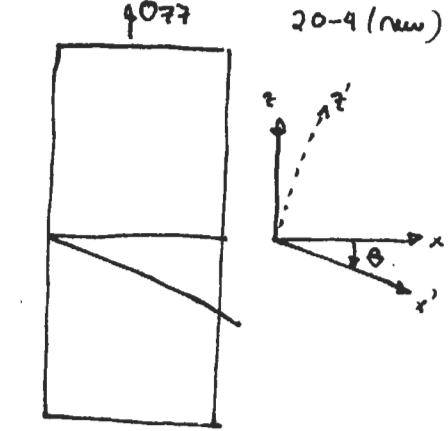
Force equilibrium for cut bar, areas are

A and A'

$$A' \cdot \cos \theta = A$$

### Horizontal

$$\sigma_{xx} = 0 \Rightarrow F_x = 0$$



$$F_x = 0 = \sigma_{x'z'} \cos \theta - \sigma_{zz'} \sin \theta$$

$$\sigma_{zz'} = \cot \theta \cdot \sigma_{x'z'}$$

### Vertical

$$F_z = A \cdot \sigma_{zz} = A' (\sigma_{x'z'} \sin \theta + \sigma_{zz'} \cos \theta) = A' (\sigma_{x'z'} \sin \theta + \cos \theta \cdot \cot \theta \cdot \sigma_{x'z'})$$

$$A' \cos \theta$$

$$A' \cos \theta \cdot \sigma_{zz} = A' \sigma_{x'z'} (\sin \theta + \cos \theta \cdot \cot \theta)$$

$$\sigma_{x'z'} = \sigma_{zz} \frac{\tan \theta}{\sin \theta + \cos \theta \cdot \cot \theta} = \sigma_{zz} \frac{1}{\tan \theta + \cot \theta}$$

$$\tan \theta + \cot \theta = \frac{\sin \theta}{\cos \theta} + \frac{\cos \theta}{\sin \theta} = \frac{\sin^2 \theta + \cos^2 \theta}{\cos \theta \sin \theta} = \frac{1}{\cos \theta \sin \theta}$$

$$\sigma_{x'z'} = \sigma_{zz} \cdot \cos \theta \cdot \sin \theta \quad \sigma_{zz'} = \frac{\cos \theta}{\sin \theta} \sigma_{x'z'} = \sigma_{zz} \cdot \cot^2 \theta$$

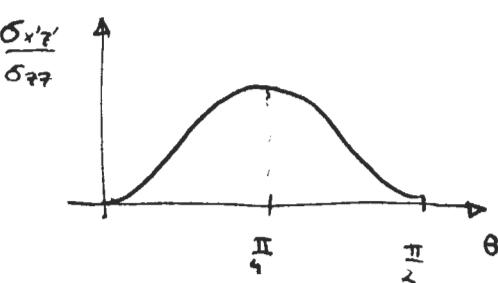
What is max  $\sigma_{x'z'}$ ?

$$\frac{\partial \sigma_{x'z'}}{\partial \theta} = \sigma_{zz} (\sin^2 \theta - \cos^2 \theta) = 0; \theta = \frac{\pi}{4}$$

Max shear stress for  $\theta = \frac{\pi}{4}$

$$\sigma_{x'z'} = \sigma_{zz} \left( \frac{1}{2} f_2 \right) \left( \frac{1}{2} f_2 \right) = \frac{\sigma_{zz}}{2}$$

$$\sigma_{zz'} = \frac{\sigma_{zz}}{2}$$



For  $\theta = \frac{\pi}{4}$

$\sigma_{x'z'} = \frac{1}{2} \sigma_{xz}$  : maximum resolved shear stress.

Note 1 stresses depend on coordinate system.

Note 2 Because  $\sigma_{ij}$  is symmetric:

$$\sigma_{ij} = \sigma_{ji}$$

It is diagonalizable; i.e. there exists an orthogonal coordinate system in which

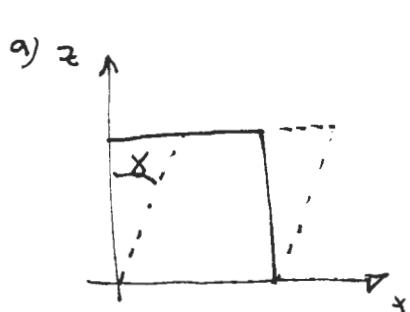
$\sigma_{ij}$  is diagonal

(Theorem from  
linear algebra)

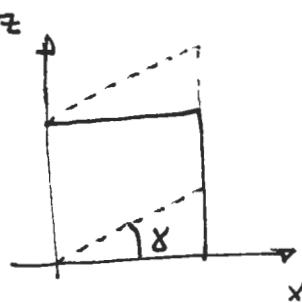
### Deformation

$$\begin{array}{ccc} P & \frac{\Delta V}{V} & \rightarrow \text{Hydrodynamics} \\ \downarrow & \uparrow & \\ \sigma_{ij} & \epsilon_{ij} & \rightarrow \text{Elasticity theory.} \end{array}$$

Form a shear deformation



Can perform the same deformation this way.



These two states should represent the same deformation  
only difference is a rotation of the coordinate system.  
(Should be Galilean invariant)

Our description of deformation should give the same shear deformation in both cases

$(x, y, z)$  displaced by  $u_x, u_y, u_z \quad u_x(x, y, z)$

case a)

$$u_x = \gamma z$$

case b)  $u_z = \gamma x$

Define deformation through changes in  $u_x, u_y, u_z$

$$\epsilon_{xz} = \frac{1}{2} \left( \frac{\partial u_z}{\partial x} + \frac{\partial u_x}{\partial z} \right)$$

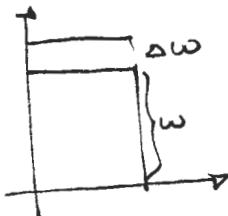
This ensures correct "rotational" transformation property.

Q: This is our definition of shear deformation:

a)  $\epsilon_{xz} = \frac{1}{2} \gamma$

b)  $\epsilon_{xz} = \frac{1}{2} \gamma$

From tensile deformation



$$\epsilon_{zz} = \frac{\Delta w}{w} = \frac{\partial u_z}{\partial z} = \frac{1}{2} \left( \frac{\partial u_z}{\partial z} + \frac{\partial u_z}{\partial z} \right)$$

General

$$\epsilon_{ij} = \frac{1}{2} \left( \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right)$$

By definition

$$\epsilon_{ij} = \epsilon_{ji}$$

We also observe that the strain  $\epsilon_{ij}$  depends on the choice of coordinate systems

We note that

$$\frac{\Delta V}{V} = \epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz} : \underline{\text{invariant}}$$


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The general link between  $\sigma_{ij}$  and  $\epsilon_{ij}$  in linear elasticity theory is

$$\sigma_{ij} = \sum_{kl} C_{ijkl} \epsilon_{kl}$$

↑ 81 indices

where

$$\left. \begin{aligned} C_{ijkl} &= C_{jikl} \\ C_{ijkl} &= C_{klij} \end{aligned} \right\} \text{symmetric}$$

Isotropic behavior:

Same properties in all directions

two independent coefficients

## Cubic crystals :

20-8

elasticity may be anisotropic

use different, 6 - component - scheme

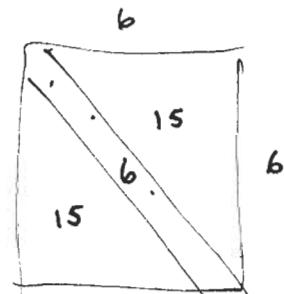
$i,j$	11	22	33	23	31	12
$k$	1	2	3	4	5	6

$$C_{11} = C_{11\ 11}$$

$$C_{12} = C_{11\ 22}$$

$$C_{44} = C_{23\ 23}$$

For cubic crystals, there are  $6 + 15$  components in  $C$



## Isotropic materials

$$\epsilon_{xx} = \frac{1}{E} (\sigma_{xx} - \nu(\sigma_{yy} + \sigma_{zz}))$$

$$\epsilon_{yy} = \frac{1}{E} (\sigma_{yy} - \nu(\sigma_{xx} + \sigma_{zz}))$$

$$\epsilon_{xy} = \frac{1}{2G} \sigma_{xy} \text{ etc.}$$

$E$  : Young's modulus

$G$  (often  $\mu$ ) : Shear modulus

$\nu$  : Poisson's ratio

Demand that constants are independent of rotation of coordinate system:

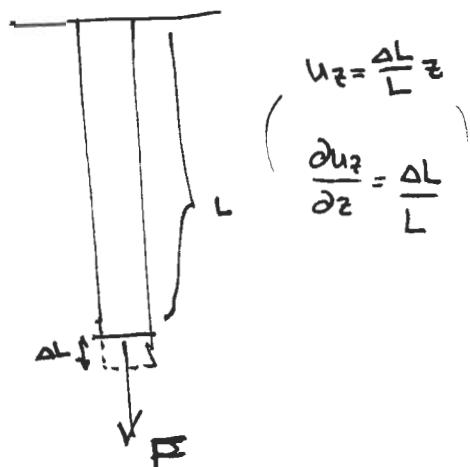
$$G = \frac{E}{2(1+\nu)}$$

### Interpretation of elastic constants

$$\begin{aligned}\epsilon_{zz} &= \frac{1}{2} \left( \frac{\partial u_z}{\partial z}, \frac{\partial u_z}{\partial z} \right) = \frac{\partial u_z}{\partial z} \\ &= \frac{\Delta L}{L} = \frac{1}{E} ( \sigma_{zz} - \nu \overbrace{(\sigma_{xx} + \sigma_{yy})}^{0!} ) \\ &= \frac{1}{E} \sigma_{zz} = \frac{1}{E} \rightarrow \frac{F}{A}\end{aligned}$$

$\frac{F}{A} = E \frac{\Delta L}{L}$

Hooke's law



$$F = (E \frac{A}{L}) \Delta L \quad (\text{Hooke's law for an elastic material})$$

$k$   $\rightarrow$  spring constant.

$$\epsilon_{xx} = \frac{1}{E} (\sigma_{xx} - \nu (\sigma_{yy} + \sigma_{zz})) = - \frac{\nu}{E} \sigma_{zz}$$

$$\frac{1}{E} \sigma_{zz} = \frac{\Delta L}{L}$$

$$\epsilon_{xx} = - \nu \frac{\Delta L}{L} = \frac{\Delta x}{x}$$

$$-\frac{\epsilon_{xx}}{\epsilon_{zz}} = \nu$$

"infrinitely compressible"  
 $-1 < \nu < \frac{1}{2}$

"incompressible": no change in volume.

20-10

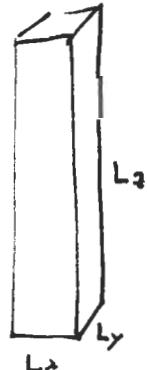
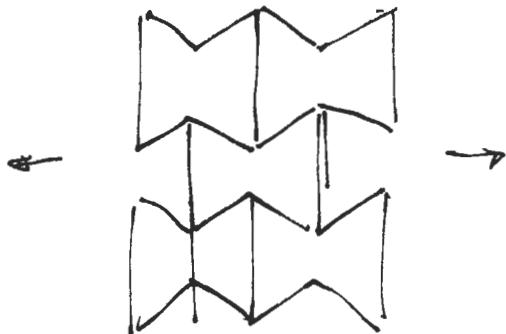
possible values for  $\nu$

For almost all materials:  $0 < \nu < 0.5$ .

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How could a  $\nu < 0$  material behave?

How can you make one?



What does  $\nu = \frac{1}{2}$  mean?

$$\epsilon_{xx} = \frac{\Delta L_x}{L_x} = -\nu \frac{\Delta L_z}{L_z} = -\frac{1}{2} \frac{\Delta L_z}{L_z}$$

$$\epsilon_{yy} = -\nu \frac{\Delta L_z}{L_z} = \frac{\Delta L_x}{L_x} = -\frac{1}{2} \frac{\Delta L_z}{L_z}$$

$$\frac{\Delta V}{V} = \underbrace{\epsilon_{xx} + \epsilon_{yy} + \epsilon_{zz}}_{=0} = (1-2\nu) \epsilon_{zz} = 0$$

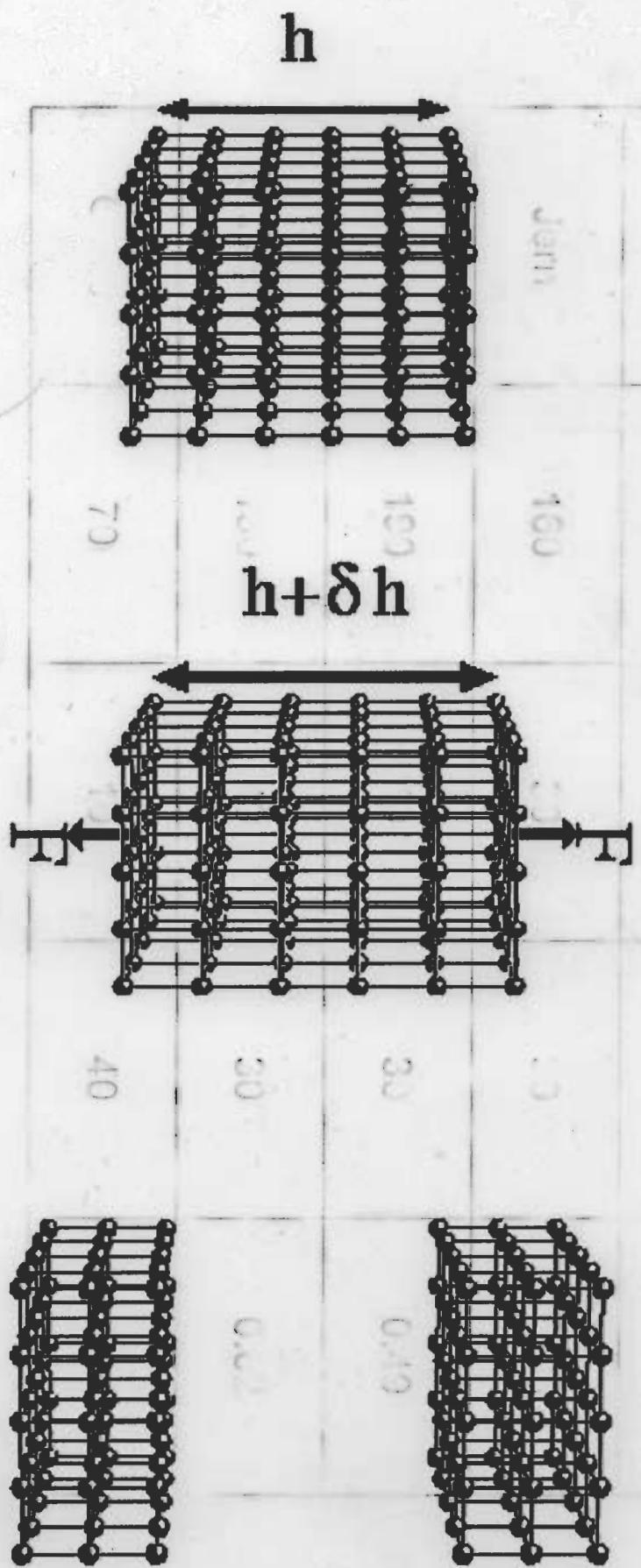
$$\nu = \frac{1}{2}$$

~~$\frac{\Delta V}{V} = \epsilon_{zz}$~~

~~with~~

Materiale	E (GPa)	E/5 (GPa)	Teoretisk styrke (GPa)	Praktisk styrke (GPa)
Jern	160	30	30	0.85
Kobber	190	40	30	0.49
Silisium	180	40	30	0.62
Glass	70	10	40	0.02

Teoretisk styrke funnet ved realistiske modeller for atomær binding.



Ten  
h: skjærlagets dybde, distansen fra bunn til forstørrelse

## Plastic crystals - dislocations

Brittle materials

→ fracture formation

→ fracture movement.

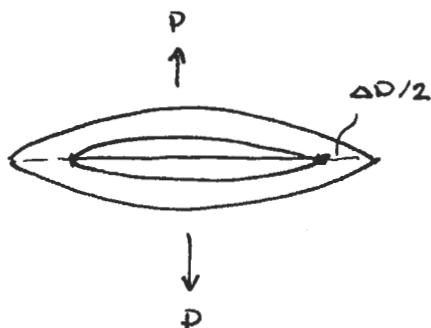
Grieffith theory (1920)

Assume there is a crack in the material

The fracture may open under external load

- opens
- form new surface

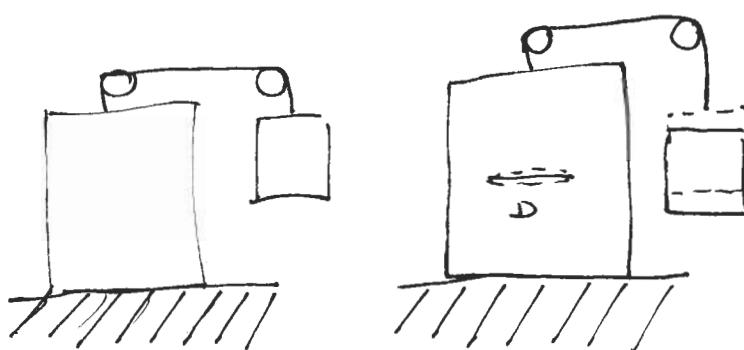
Assume change in fracture length is  $\Delta D$



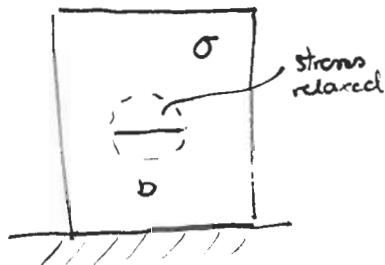
$$\text{Energy } 2\gamma \Delta D - \Delta E_{el}$$

↑ reduction of elastic energy.

More detail



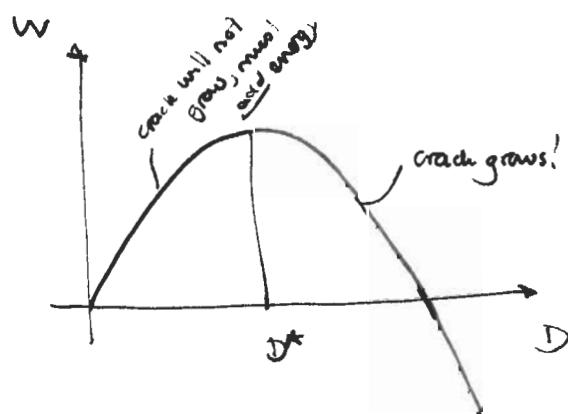
What is the energy difference,  
corresponding to the work?  $W$



$2\gamma D$ : surface energy  $\gamma$

$$-\frac{\sigma^2}{2G} D^2 \text{ relaxation of elastic energy}$$

$$W = 2\gamma D - \frac{\sigma^2}{2G} D^2$$



Critical  $D^*$

$$\frac{\partial W}{\partial D} = 0$$

$$2\gamma - \frac{\sigma^2}{G} D^* = 0$$

$$D^* = \frac{2G\gamma}{\sigma^2}$$

Or

$$\sigma^* = \sqrt{\frac{2\gamma G}{D}} : \text{critical load, gives length of crack}$$

Graffith criterion

D.



D.

"Justification" of  $\frac{\sigma^2}{2G} D^2$  term.

$\sigma D$  is force on surface.

$\frac{1}{2} \frac{\sigma D}{G}$  is distance, because

$$\sigma = G \frac{\partial u}{\partial y}$$

$\frac{\sigma}{G} = \frac{\partial u}{\partial y}$ ; total displacement at center

$$\frac{\sigma}{G} D = u$$

avg displacement is  $\frac{1}{2} \frac{\sigma}{G} D$

$$W = \frac{1}{2} \frac{\sigma}{G} D \cdot \sigma D = \frac{1}{2} \frac{\sigma^2}{G} D^2$$